

R4 Solution. Grouted cavity retaining wall

The design value of the moment applied,

$$M_{Ed} = \gamma_{G,sup} G_k + \gamma_Q Q_{k,1} = (1,35 \times 7,9 \times (1,8/3)) + (1,5 \times 4,9 \times (1,8/2)) = 13,01 \text{ kNm}$$

Serviceability limiting span [BS EN 1996-1-1 Table 5.2]

Limiting span to effective depth ratio is 18

Hence effective depth must exceed $l_{ef}/18 = 1800/18 = 100 \text{ mm}$

f_b is the normalised strength of a masonry unit.

For a Group 1 440 x 215 x 100 mm dense aggregate masonry unit

$$f_b = 7 \times 1,38 \times 1 = 9,66 \text{ N/mm}^2 \text{ [Annex A of BS EN 772-1]}$$

M6 mortar to be used

Characteristic compressive strength of the masonry $f_k = 0,75(9,66)^{0,7}(6)^{0,3} = 6,3 \text{ N/mm}^2$

[Equation 3.1 of BS EN 1996-1-1 and National Annex to BS EN 1996-1-1]

For the higher level of site supervision required for reinforced masonry $\gamma_m = 2$

Design compressive strength of the masonry $f_d = 6,3/2 = 3,15 \text{ N/mm}^2$

$$Q = \frac{M_{Ed}}{bd^2} = \frac{13,01 \times 10^6}{10^3 \times 165^2} = 0,478 \text{ [PD 6697:2010]}$$

$$Q = 2c(1 - c)f_d = 0,478$$

$$6,3c^2 - 6,3c + 0,478 = 0$$

$$c = 0,92$$

Hence lever arm $= 0,92 \times 165 = 152 \text{ mm}$

$$A_s = \frac{M_{Ed}}{f_{yd} Z}$$

$$f_{yd} = 500/1,15 = 435 \text{ N/mm}^2$$

$$A_s = \frac{13,01 \times 10^6}{435 \times 152} = 197 \text{ mm}^2$$

Therefore the steel selected is adequate.

Check limit on compressive strength of concrete masonry unit [Equation 6.24 of BS EN 1996-1-1]

$$M_{Rd} \leq 0,4f_d b d^2 = 0,4 \times 3,15 \times 10^3 \times 165^2 \times 10^{-6} = 34 \text{ kNm} \quad \text{Therefore O.K.}$$

Check minimum brick strength required

$$F_b = 9,66 = \text{brick strength} \times 0,85 \times 1,0 = 11,36 \text{ N/mm}^2$$

Therefore the brick selected is O.K.

Check shear capacity

Design value of the shear load $V_{Ed} = 7,9 \times 1,35 + 4,9 \times 1,5 = 18.01 \text{ kN}$ per m run of wall

$$f_{vd} = \frac{(0,35 + 17,5\rho)}{\gamma_m} \quad [\text{Equation J1 of BS EN 1996-1-1}]$$

$$\rho = \frac{A_s}{bd} = \frac{252}{10^3 \times 165} = 0,0015$$

$$\gamma_{M \text{ shear}} = 2.0$$

$$f_{vd} = \frac{(0,35 + (17,5 \times 0,0015))}{2} = 0,19 \text{ N/mm}^2$$

$$V_{Rd} = f_{vd} b d = 0,19 \times 10^3 \times 165 \times 10^{-3} = 31,35 \text{ kN}$$

Therefore the shear resistance is adequate.